

STRUCTURAL DESIGN OF UNREINFORCED COMPOSITE MASONRY

TEK 16-2B
Structural (2001)

Keywords: allowable loads, bonding, concrete brick, composite walls, flexural strength, multi-wythe walls

INTRODUCTION

Concrete masonry offers many textures, colors and sizes, along with choices in bond patterns and joint treatment making it an excellent choice for exterior and interior walls in residential, commercial and public buildings. Concrete brick can be used in both structural and veneer applications and is economical, durable, easy to maintain, fire resistant, and reduces sound transmission.

Multi-wythe masonry walls are classified as either composite or noncomposite depending on how the wythes interact. Connections between wythes of composite walls are designed to transfer stresses between the wythes, allowing the wythes to act as a single member in resisting loads. In contrast, for noncomposite or cavity walls each wythe individually resists the loads imposed on it. Concrete brick are used both in composite walls and as nonloadbearing veneer in cavity wall construction. Requirements for concrete brick veneers are summarized in *Concrete Masonry Veneers*, TEK 3-6A (ref. 1).

Standard Specification for Concrete Building Brick, ASTM C 55 (ref. 2), governs concrete brick and similar solid units. C 55 requirements are summarized in *ASTM Specifications for Concrete Masonry Units*, TEK 1-1C (ref. 3).

STRUCTURAL DESIGN METHODS

Composite wall structural design requirements are contained in *Building Code Requirements for Masonry Structures* (ref. 4) and the *International Building Code* (ref. 5).

Allowable stress design of unreinforced composite walls is typically governed by the flexural tensile capacity of the masonry system (see Table 1), although compression and shear must also be checked. Shear stress in the plane of interface between wythes and collar joints is limited to 5 psi (34.5 kPa) for mortared collar joints; 10 psi (68.9 kPa) for grouted collar joints; and the square root of the unit compressive strength of the header (over the net area of the header) for headers.

Tables 2 through 13, for lateral loads with or without concentric axial loads (see Figure 1), are based on Chapter 2, Allowable Stress Design, of *Building Code Require-*

- ments for Masonry Structures* (ref. 4) and the following:
- (1) specified compressive strength of masonry, $f'_m = 1500$ psi (10.3 MPa),
 - (2) section modulus based on the minimum net area of the composite wall cross section,
 - (3) faceshell and web dimensions based on ASTM C 90 (ref. 6) minimum requirements for hollow units,
 - (4) loads include $1/3$ increase in allowable stress for load combinations including wind or seismic (where $1/3$ increase does not apply, multiply the Table values by $3/4$), and
 - (5) allowable tensile stress in masonry, F_t , for hollow ungrouted concrete masonry normal to the bed joints is as noted in Table footnotes.

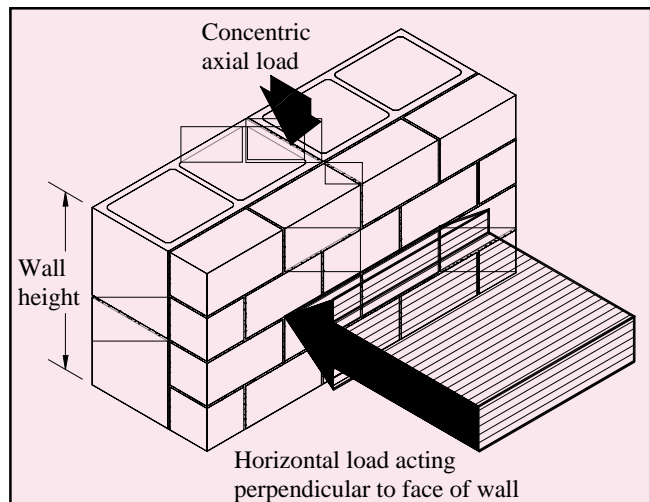


Figure 1—Concentric Axial and Lateral Loading

Table 1—Allowable Flexural Tension, psi (kPa) (ref. 4)

| Mortar type: | Portland cement/lime or mortar cement | | Masonry cement or air entrained Portland cement/lime | |
|--|---------------------------------------|----------|--|----------|
| | M or S | N | M or S | N |
| Solid units: | | | | |
| Normal to bed joints | 40 (276) | 30 (207) | 24 (166) | 15 (103) |
| Parallel to bed joints in running bond | 80 (552) | 60 (414) | 48 (331) | 30 (207) |
| UngROUTED hollow units: | | | | |
| Normal to bed joints | 25 (172) | 19 (131) | 15 (103) | 9 (62) |
| Parallel to bed joints in running bond | 50 (345) | 38 (262) | 30 (207) | 19 (131) |

Type N Portland Cement/Lime or Mortar Cement^a

Table 2—Maximum Horizontal Load in psf (kPa) on Eight-inch (203 mm) Thick Composite Wall

8-in. (203 mm) composite wall composed of 4-in. (102 mm) concrete brick and 4-in. (102 mm) hollow concrete masonry units

| Wall height, ft (m) | Axial load, lb/ft (kN/m) | | |
|------------------------|--------------------------|--------------|-----------------------|
| | 0 (0) | 1,000 (14.6) | 2,000 (29.2) |
| 8 (2.4) | 16 (0.80) | 26 (1.2) | 36 ^c (1.7) |
| 9 (2.7) | 13 (0.63) | 21 (1.0) | 29 ^c (1.3) |
| 10 (3.0) | 10 (0.51) | 17 (0.82) | 23 (1.1) |
| 11 (3.4) | 8.9 (0.42) | 14 (0.68) | 19 (0.93) |
| 12 (3.7) | 7.5 (0.35) | 11 (0.57) | 16 (0.78) |
| 13 (4.0) | 6.4 (0.30) | 10 (0.48) | 14 (0.66) |
| 14 (4.3) | 5.5 (0.26) | 8.8 (0.42) | 12 (0.57) |
| 15 (4.6) | 4.8 (0.22) | 7.6 (0.36) | 10 (0.50) |
| 16 (4.9) | 4.2 (0.20) | 6.7 (0.32) | 9.2 (0.44) |

Type N Masonry Cement or Air Entrained Portland Cement/Lime Mortar^b

Table 5—Maximum Horizontal Load in psf (kPa) on Eight-inch (203 mm) Thick Composite Wall

8-in. (203 mm) composite wall composed of 4-in. (102 mm) concrete brick and 4-in. (102 mm) hollow concrete masonry units

| Wall height, ft (m) | Axial load, lb/ft (kN/m) | | |
|------------------------|--------------------------|--------------|--------------|
| | 0 (0) | 1,000 (14.6) | 2,000 (29.2) |
| 8 (2.4) | 7.9 (0.38) | 18 (0.86) | 28 (1.3) |
| 9 (2.7) | 6.3 (0.30) | 14 (0.68) | 22 (1.0) |
| 10 (3.0) | 5.1 (0.24) | 11 (0.55) | 18 (0.86) |
| 11 (3.4) | 4.2 (0.20) | 9.5 (0.45) | 14 (0.71) |
| 12 (3.7) | 3.5 (0.16) | 8.0 (0.38) | 12 (0.59) |
| 13 (4.0) | 3.0 (0.14) | 6.8 (0.32) | 10 (0.50) |
| 14 (4.3) | 2.6 (0.12) | 5.9 (0.28) | 9.2 (0.43) |
| 15 (4.6) | 2.3 (0.10) | 5.1 (0.24) | 8.0 (0.38) |
| 16 (4.9) | 2.0 (0.09) | 4.5 (0.21) | 7.0 (0.33) |

Table 3—Maximum Horizontal Load in psf (kPa) on Ten-inch (254 mm) Thick Composite Wall

10-in. (254 mm) composite wall composed of 4-in. (102 mm) concrete brick and 6-in. (152 mm) hollow concrete masonry units

| Wall height, ft (m) | Axial load, lb/ft (kN/m) | | |
|------------------------|--------------------------|--------------|-----------------------|
| | 0 (0) | 1,000 (14.6) | 2,000 (29.2) |
| 8 (2.4) | 24 (1.1) | 38 (1.8) | 51 ^c (2.4) |
| 9 (2.7) | 19 (0.93) | 30 (1.4) | 41 ^c (1.9) |
| 10 (3.0) | 15 (0.75) | 24 (1.1) | 33 (1.5) |
| 11 (3.4) | 13 (0.62) | 20 (0.97) | 27 (1.3) |
| 12 (3.7) | 11 (0.52) | 17 (0.81) | 23 (1.1) |
| 13 (4.0) | 9.4 (0.44) | 14 (0.69) | 19 (0.94) |
| 14 (4.3) | 8.1 (0.38) | 12 (0.59) | 16 (0.81) |
| 15 (4.6) | 7.0 (0.33) | 10 (0.52) | 14 (0.70) |
| 16 (4.9) | 6.2 (0.29) | 9.6 (0.45) | 13 (0.62) |
| 17 (5.2) | 5.5 (0.26) | 8.5 (0.40) | 11 (0.55) |
| 18 (5.5) | 4.9 (0.23) | 7.6 (0.36) | 10 (0.49) |
| 19 (5.8) | 4.4 (0.21) | 6.8 (0.32) | 9.2 (0.44) |
| 20 (6.1) | 4.0 (0.18) | 6.1 (0.29) | 8.3 (0.39) |

Table 6—Maximum Horizontal Load in psf (kPa) on Ten-inch (254 mm) Thick Composite Wall

10-in. (254 mm) composite wall composed of 4-in. (102 mm) concrete brick and 6-in. (152 mm) hollow concrete masonry units

| Wall height, ft (m) | Axial load, lb/ft (kN/m) | | |
|------------------------|--------------------------|--------------|--------------|
| | 0 (0) | 1,000 (14.6) | 2,000 (29.2) |
| 8 (2.4) | 11 (0.56) | 25 (1.2) | 38 (1.8) |
| 9 (2.7) | 9.3 (0.44) | 20 (0.95) | 30 (1.4) |
| 10 (3.0) | 7.5 (0.35) | 16 (0.77) | 24 (1.1) |
| 11 (3.4) | 6.2 (0.29) | 13 (0.64) | 20 (0.98) |
| 12 (3.7) | 5.2 (0.24) | 11 (0.53) | 17 (0.82) |
| 13 (4.0) | 4.4 (0.21) | 9.6 (0.45) | 14 (0.70) |
| 14 (4.3) | 3.8 (0.18) | 8.3 (0.39) | 12 (0.60) |
| 15 (4.6) | 3.3 (0.15) | 7.2 (0.34) | 11 (0.52) |
| 16 (4.9) | 2.9 (0.14) | 6.3 (0.30) | 9.7 (0.46) |
| 17 (5.2) | 2.6 (0.12) | 5.6 (0.26) | 8.6 (0.41) |
| 18 (5.5) | 2.3 (0.11) | 5.0 (0.23) | 7.7 (0.36) |
| 19 (5.8) | 2.1 (0.099) | 4.5 (0.21) | 6.9 (0.33) |
| 20 (6.1) | 1.9 (0.090) | 4.0 (0.19) | 6.2 (0.29) |

Table 4—Maximum Horizontal Load in psf (kPa) on Twelve-inch (305 mm) Thick Composite Wall

12-in. (305 mm) composite wall composed of 4-in. (102 mm) concrete brick and 8-in. (203 mm) hollow concrete masonry units

| Wall height, ft (m) | Axial load, lb/ft (kN/m) | | |
|------------------------|--------------------------|-----------------------|-----------------------|
| | 0 (0) | 1,000 (14.6) | 2,000 (29.2) |
| 8 (2.4) | 43 (2.0) | 65 ^c (3.1) | 88 ^c (4.2) |
| 9 (2.7) | 34 (1.6) | 52 ^c (2.4) | 69 ^c (3.3) |
| 10 (3.0) | 28 (1.3) | 42 (2.0) | 56 ^c (2.6) |
| 11 (3.4) | 23 (1.1) | 34 (1.6) | 46 ^c (2.2) |
| 12 (3.7) | 19 (0.93) | 29 (1.4) | 39 ^c (1.8) |
| 13 (4.0) | 16 (0.79) | 24 (1.1) | 33 (1.5) |
| 14 (4.3) | 14 (0.68) | 21 (1.0) | 28 (1.3) |
| 15 (4.6) | 12 (0.59) | 18 (0.89) | 25 (1.1) |
| 16 (4.9) | 10 (0.52) | 16 (0.78) | 22 (1.0) |
| 17 (5.2) | 9.7 (0.46) | 14 (0.69) | 19 (0.93) |
| 18 (5.5) | 8.6 (0.41) | 13 (0.62) | 17 (0.83) |
| 19 (5.8) | 7.7 (0.37) | 11 (0.55) | 15 (0.74) |
| 20 (6.1) | 7.0 (0.33) | 10 (0.50) | 14 (0.67) |

Table 7—Maximum Horizontal Load in psf (kPa) on Twelve-inch (305 mm) Thick Composite Wall

12-in. (305 mm) composite wall composed of 4-in. (102 mm) concrete brick and 8-in. (203 mm) hollow concrete masonry units

| Wall height, ft (m) | Axial load, lb/ft (kN/m) | | |
|------------------------|--------------------------|--------------|-----------------------|
| | 0 (0) | 1,000 (14.6) | 2,000 (29.2) |
| 8 (2.4) | 20 (0.99) | 42 (2.0) | 65 ^c (3.1) |
| 9 (2.7) | 16 (0.78) | 33 (1.6) | 51 ^c (2.4) |
| 10 (3.0) | 13 (0.63) | 27 (1.3) | 41 (1.9) |
| 11 (3.4) | 10 (0.52) | 22 (1.0) | 34 (1.6) |
| 12 (3.7) | 9.2 (0.44) | 19 (0.91) | 28 (1.3) |
| 13 (4.0) | 7.8 (0.37) | 16 (0.77) | 24 (1.1) |
| 14 (4.3) | 6.8 (0.32) | 14 (0.67) | 21 (1.0) |
| 15 (4.6) | 5.9 (0.28) | 12 (0.58) | 18 (0.88) |
| 16 (4.9) | 5.2 (0.24) | 10 (0.51) | 16 (0.77) |
| 17 (5.2) | 4.6 (0.21) | 9.5 (0.45) | 14 (0.69) |
| 18 (5.5) | 4.1 (0.19) | 8.5 (0.40) | 12 (0.61) |
| 19 (5.8) | 3.7 (0.17) | 7.6 (0.36) | 11 (0.55) |
| 20 (6.1) | 3.3 (0.15) | 6.9 (0.32) | 10 (0.49) |

^a $F_v = 19$ psi (131 kPa)

^b $F_v = 9$ psi (62 kPa)

^c Shear exceeds the allowable for collar joints crossed by connecting masonry headers, therefore wythes of these walls must be connected via a collar joint filled with mortar or grout and connected by wall ties.

Type M or S Portland Cement/Lime or Mortar Cement^c

Table 8—Maximum Horizontal Load in psf (kPa) on Eight-inch (203 mm) Thick Composite Wall

8-in. (203 mm) composite wall composed of 4-in. (102 mm) concrete brick and 4-in. (102 mm) hollow concrete masonry units

| Wall height, ft (m) | Axial load, lb/ft (kN/m) | | |
|------------------------|--------------------------|-----------------------|-----------------------|
| | 0 (0) | 1,000 (14.6) | 2,000 (29.2) |
| 8 (2.4) | 22 (1.0) | 32 ^e (1.5) | 42 ^e (2.0) |
| 9 (2.7) | 17 (0.83) | 25 (1.2) | 33 ^e (1.5) |
| 10 (3.0) | 14 (0.67) | 20 (0.98) | 27 ^e (1.2) |
| 11 (3.4) | 11 (0.55) | 17 (0.81) | 22 (1.0) |
| 12 (3.7) | 9.8 (0.47) | 14 (0.68) | 18 (0.89) |
| 13 (4.0) | 8.4 (0.40) | 12 (0.58) | 16 (0.76) |
| 14 (4.3) | 7.2 (0.34) | 10 (0.50) | 13 (0.66) |
| 15 (4.6) | 6.3 (0.30) | 9.1 (0.43) | 12 (0.57) |
| 16 (4.9) | 5.5 (0.26) | 8.0 (0.38) | 10 (0.50) |

Type M or S Masonry Cement or Air Entrained Portland Cement/Lime Mortar^d

Table 11—Maximum Horizontal Load in psf (kPa) on Eight-inch (203 mm) Thick Composite Wall

8-in. (203 mm) composite wall composed of 4-in. (102 mm) concrete brick and 4-in. (102 mm) hollow concrete masonry units

| Wall height, ft (m) | Axial load, lb/ft (kN/m) | | |
|------------------------|--------------------------|--------------|-----------------------|
| | 0 (0) | 1,000 (14.6) | 2,000 (29.2) |
| 8 (2.4) | 13 (0.63) | 23 (1.1) | 33 ^e (1.5) |
| 9 (2.7) | 10 (0.50) | 18 (0.88) | 26 (1.2) |
| 10 (3.0) | 8.5 (0.40) | 14 (0.71) | 21 (1.0) |
| 11 (3.4) | 7.0 (0.33) | 12 (0.59) | 17 (0.84) |
| 12 (3.7) | 5.9 (0.28) | 10 (0.49) | 14 (0.71) |
| 13 (4.0) | 5.0 (0.24) | 8.8 (0.42) | 12 (0.60) |
| 14 (4.3) | 4.3 (0.20) | 7.6 (0.36) | 10 (0.52) |
| 15 (4.6) | 3.8 (0.18) | 6.6 (0.31) | 9.5 (0.45) |
| 16 (4.9) | 3.3 (0.15) | 5.8 (0.27) | 8.3 (0.39) |

Table 9—Maximum Horizontal Load in psf (kPa) on Ten-inch (254 mm) Thick Composite Wall

10-in. (254 mm) composite wall composed of 4-in. (102 mm) concrete brick and 6-in. (152 mm) hollow concrete masonry units

| Wall height, ft (m) | Axial load, lb/ft (kN/m) | | |
|------------------------|--------------------------|-----------------------|-----------------------|
| | 0 (0) | 1,000 (14.6) | 2,000 (29.2) |
| 8 (2.4) | 32 (1.5) | 46 ^e (2.2) | 59 ^e (2.8) |
| 9 (2.7) | 25 (1.2) | 36 (1.7) | 47 ^e (2.2) |
| 10 (3.0) | 20 (0.99) | 29 (1.4) | 38 ^e (1.8) |
| 11 (3.4) | 17 (0.82) | 24 (1.1) | 31 ^e (1.5) |
| 12 (3.7) | 14 (0.69) | 20 (0.98) | 26 (1.2) |
| 13 (4.0) | 12 (0.59) | 17 (0.83) | 22 (1.0) |
| 14 (4.3) | 10 (0.50) | 15 (0.72) | 19 (0.93) |
| 15 (4.6) | 9.3 (0.44) | 13 (0.62) | 17 (0.81) |
| 16 (4.9) | 8.1 (0.38) | 11 (0.55) | 14 (0.71) |
| 17 (5.2) | 7.2 (0.34) | 10 (0.48) | 13 (0.63) |
| 18 (5.5) | 6.4 (0.30) | 9.1 (0.43) | 11 (0.56) |
| 19 (5.8) | 5.8 (0.27) | 8.2 (0.39) | 10 (0.50) |
| 20 (6.1) | 5.2 (0.24) | 7.4 (0.35) | 9.6 (0.45) |

Table 12—Maximum Horizontal Load in psf (kPa) on Ten-inch (254 mm) Thick Composite Wall

10-in. (254 mm) composite wall composed of 4-in. (102 mm) concrete brick and 6-in. (152 mm) hollow concrete masonry units

| Wall height, ft (m) | Axial load, lb/ft (kN/m) | | |
|------------------------|--------------------------|--------------|-----------------------|
| | 0 (0) | 1,000 (14.6) | 2,000 (29.2) |
| 8 (2.4) | 19 (0.93) | 33 (1.5) | 46 ^e (2.2) |
| 9 (2.7) | 15 (0.73) | 26 (1.2) | 36 (1.7) |
| 10 (3.0) | 12 (0.59) | 21 (1.0) | 29 (1.4) |
| 11 (3.4) | 10 (0.49) | 17 (0.83) | 24 (1.1) |
| 12 (3.7) | 8.7 (0.41) | 14 (0.70) | 20 (0.99) |
| 13 (4.0) | 7.4 (0.35) | 12 (0.60) | 17 (0.84) |
| 14 (4.3) | 6.4 (0.30) | 10 (0.51) | 15 (0.73) |
| 15 (4.6) | 5.6 (0.26) | 9.4 (0.45) | 13 (0.63) |
| 16 (4.9) | 4.9 (0.23) | 8.3 (0.39) | 11 (0.55) |
| 17 (5.2) | 4.3 (0.20) | 7.3 (0.35) | 10 (0.49) |
| 18 (5.5) | 3.9 (0.18) | 6.5 (0.31) | 9.2 (0.44) |
| 19 (5.8) | 3.5 (0.16) | 5.9 (0.28) | 8.3 (0.39) |
| 20 (6.1) | 3.1 (0.14) | 5.3 (0.25) | 7.5 (0.35) |

Table 10—Maximum Horizontal Load in psf (kPa) on Twelve-inch (305 mm) Thick Composite Wall

12-in. (305 mm) composite wall composed of 4-in. (102 mm) concrete brick and 8-in. (203 mm) hollow concrete masonry units

| Wall height, ft (m) | Axial load, lb/ft (kN/m) | | |
|------------------------|--------------------------|-----------------------|------------------------|
| | 0 (0) | 1,000 (14.6) | 2,000 (29.2) |
| 8 (2.4) | 57 ^e (2.7) | 79 ^e (3.8) | 101 ^e (4.8) |
| 9 (2.7) | 45 (2.1) | 62 ^e (3.0) | 80 ^e (3.8) |
| 10 (3.0) | 36 (1.7) | 51 ^e (2.4) | 65 ^e (3.1) |
| 11 (3.4) | 30 (1.4) | 42 ^e (2.0) | 53 ^e (2.5) |
| 12 (3.7) | 25 (1.2) | 35 (1.6) | 45 ^e (2.1) |
| 13 (4.0) | 21 (1.0) | 30 (1.4) | 38 ^e (1.8) |
| 14 (4.3) | 18 (0.89) | 26 (1.2) | 33 ^e (1.5) |
| 15 (4.6) | 16 (0.78) | 22 (1.0) | 29 (1.3) |
| 16 (4.9) | 14 (0.68) | 19 (0.95) | 25 (1.2) |
| 17 (5.2) | 12 (0.61) | 17 (0.84) | 22 (1.0) |
| 18 (5.5) | 11 (0.54) | 15 (0.75) | 20 (0.96) |
| 19 (5.8) | 10 (0.48) | 14 (0.67) | 18 (0.86) |
| 20 (6.1) | 9.2 (0.44) | 12 (0.61) | 16 (0.78) |

Table 13—Maximum Horizontal Load in psf (kPa) on Twelve-inch (305 mm) Thick Composite Wall

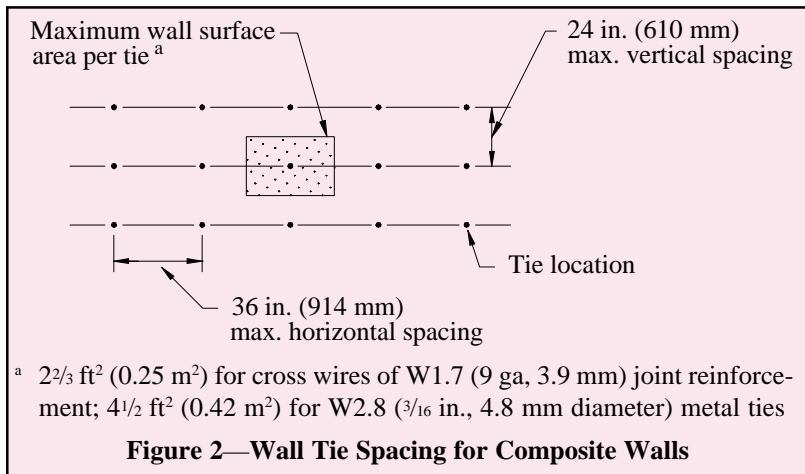
12-in. (305 mm) composite wall composed of 4-in. (102 mm) concrete brick and 8-in. (203 mm) hollow concrete masonry units

| Wall height, ft (m) | Axial load, lb/ft (kN/m) | | |
|------------------------|--------------------------|-----------------------|-----------------------|
| | 0 (0) | 1,000 (14.6) | 2,000 (29.2) |
| 8 (2.4) | 34 (1.6) | 56 ^e (2.7) | 78 ^e (3.7) |
| 9 (2.7) | 27 (1.3) | 44 (2.1) | 62 ^e (2.9) |
| 10 (3.0) | 22 (1.0) | 36 (1.7) | 50 ^e (2.4) |
| 11 (3.4) | 18 (0.87) | 30 (1.4) | 41 ^e (1.9) |
| 12 (3.7) | 15 (0.73) | 25 (1.2) | 35 (1.6) |
| 13 (4.0) | 13 (0.62) | 21 (1.0) | 29 (1.4) |
| 14 (4.3) | 11 (0.53) | 18 (0.88) | 25 (1.2) |
| 15 (4.6) | 9.8 (0.47) | 16 (0.77) | 22 (1.0) |
| 16 (4.9) | 8.6 (0.41) | 14 (0.67) | 19 (0.94) |
| 17 (5.2) | 7.6 (0.36) | 12 (0.60) | 17 (0.83) |
| 18 (5.5) | 6.8 (0.32) | 11 (0.53) | 15 (0.74) |
| 19 (5.8) | 6.1 (0.29) | 10 (0.48) | 14 (0.66) |
| 20 (6.1) | 5.5 (0.26) | 9.1 (0.43) | 12 (0.60) |

^c $F_v = 25$ psi (172 kPa)

^d $F_v = 15$ psi (103 kPa)

^e Shear exceeds the allowable for collar joints crossed by connecting masonry headers, therefore wythes of these walls must be connected via a collar joint filled with mortar or grout and connected by wall ties.



CONSTRUCTION

Concrete brick walls and wythes of concrete brick should be laid with full head and bed mortar joints. For composite construction, the collar joint (the vertical longitudinal joint between wythes of masonry) is filled with grout or mortar to allow structural interaction between the wythes.

In composite walls, *Building Code Requirements for Masonry Structures* (ref. 4) requires that concrete brick be bonded to the backup wythe using either masonry headers or wall tie and grout or mortar. These minimum requirements, described below, help ensure that composite action is present between the wythes.

When bonded using masonry headers, the headers must make up at least 4 percent of the wall surface and extend at least 3 in. (76 mm) into the backing. The shear stress developed in the masonry header is limited to the square root of the unit compressive strength of the header (in psi (MPa) over the net area of the header).

Figure 2 illustrates wall tie spacing requirements for com-

posite walls bonded with corrosion resistant ties or wire and collar joints filled with mortar or grout. Cross wires of joint reinforcement and rectangular ties are commonly used as wall ties for composite walls. Z-ties, however, are not permitted with ungrouted hollow masonry (ref. 7).

For cavity wall construction, the following construction recommendations apply:

- keep cavity substantially clean to allow free water drainage,
- install weep holes at 32 in. (813 mm) o. c.,
- install granular fill, mesh or other mortar collection device in bottom of cavity to prevent mortar droppings from blocking weep holes, and
- embed wall ties at least $1\frac{1}{2}$ in. (38 mm) into the mortar bed of solid units.

REFERENCES

1. *Concrete Masonry Veneers*, TEK 3-6A. National Concrete Masonry Association, 2000.
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